

MEMORANDUM

Project:	Land at Cranesfield, Sherborne St. John	Author:	Callum Jeffrey
Subject:	Response to LLFA Planning Comments	Checker:	Paul Daykin
Project Number	C3701	BIM Ref.	HSP2025-C3701-C&S-FN-3794
Date:	14 th January 2026	Rev.	P02

**Project Title: Land at Cranesfield, Sherborne St. John, Basingstoke, RG24 9LN
Response to LLFA comments contained within planning application consultation
response (reference 25/02051/FUL) dated 13th October 2025.**

Consultee comments reproduced and/or paraphrased in *black italicised* text.
HSP responses in **red** below.

Surface Water Flood Risk

It appears the flood risk area indicated on the mapping may underestimate the flood extent compared to observed floods. An underestimate of flood extent is not unusual due to the way the surface water flood maps are produced using course ground level and soil type data.

The above flood risk assessment concludes that the identified areas of surface water flood risk will coincide with rear gardens of the proposed properties. Further evidence is required to confirm this conclusion.

We advise that development should be located outside of identified areas of flood risk from any source and green corridors should be maintained to ensure there is not loss of flood storage and that existing flow routes are maintained.

To ensure flood risk is not increased in other areas, ground levels in identified areas of surface water flood risk should not be raised and existing overland flow paths should be maintained.

Surface Water Flood Risk mapping has been updated since the application was made. Flood risk envelopes now indicate that an isolated pocket of elevated surface water flood risk is present in the southeast corner of the site. This appears to be as a result of general overland flows/runoff from the west entering the site and ponding in a localised and distinct low point in the existing site topography. This distinct area, disconnected from the flood risk envelope to the east of the site, is inferred to be that depicted in photographs provided by local residents in their submissions to the Planning Authority.

Proposed levels will regularise terrain and remove any low points which would present a risk to the developed areas. Moreover, the proposed drainage network will intercept and attenuate contributing overland flows as they pass through the site, further alleviating flood risk.

Exceedance flows will be directed along access roads and around/away from entry points to dwellings. The general trend in landform (falling from west to east) will be maintained.

Please refer to the latest Flood Risk Assessment (HSP2021-C3701-C&S-FRAS1-250 Rev I) which includes further commentary relating to the above and also reproduces a drawing entitled Overland Flow Plan (ref. C3701-DS002). This drawing also illustrates the existing surface water flood risk in relation to the proposed layout.

Groundwater Flood Risk

The LLFA are statutory consultees with respect to surface water flood risk and surface water drainage design. Groundwater flood risk is outside our remit as planning consultee. We note the above flood risk assessment (12.3.4) recommends site investigation in the form of groundwater monitoring to assess the risk of groundwater emergence. The LLFA are unable to provide specific advice in this respect but recommend that groundwater flood risk assessment should be concluded before the application is determined.

A programme of groundwater monitoring is currently ongoing at the site. This will accurately determine groundwater levels and allow an assessment of the impact of groundwater on the proposed development. The results of this monitoring are anticipated to be available in February 2026.

Please refer to the appended Southern Testing Factual Ground Investigation Report (ref. STL J16111) for details of ground investigation/monitoring works undertaken to date.

Notwithstanding the above, it is expected that the results of the groundwater monitoring will influence the detailing of the proposals, such as the inclusion of impermeable liners, dewatering during construction, etc, rather than principle of development.

Surface Water Drainage Strategy

In principle the proposed surface water drainage strategy seems appropriate.

Site investigation (infiltration testing and groundwater monitoring) is required to confirm the assumption that infiltration drainage is not feasible. We acknowledge that it is unlikely that infiltration drainage will be viable for this site. Providing a feasible surface water drainage strategy is presented we recommend that site investigation can be provided by condition if the application is granted.

Bearing in mind this is an application for full planning permission, we consider further detail is required to provide full confidence in the feasibility of the proposed surface water drainage design.

Given the clay nature of the soils encountered on site, conventional soakaway drainage is not considered to be viable and no infiltration testing has been undertaken. This position is reinforced by information provided by Residents to the Planning Authority in their response to the planning application; namely, that the surficial soils are impermeable.

Deep borehole soakaways are also considered unviable given the soluble rock risk in the underlying chalk layers.

Please refer to the appended Southern Testing Factual Ground Investigation Report (ref. STL J16111) for further information.

Proposed Outfall

As stated in section 16.2 of the above report: the topographic survey does not include details of the location of the proposed outfall which appears to be outside of the application boundary. Third party consent may be required for the construction of the outfall. The LLFA consider topographical survey information and confirmation of required consents are required to demonstrate the feasibility of the surface water drainage design and should be submitted before we are able to provide a recommendation to the LPA.

Ordinary water course consent will be required for any proposed works affecting existing watercourses.

It has been identified during site reconnaissance, that the previously proposed outfall ditch may be located partially or wholly within third party land and within a heavily vegetated area that was considered difficult to survey. An alternative outfall location has therefore been identified.

Surface water drainage from the proposed development is now proposed to discharge indirectly to the Wey Brook, an ordinary watercourse to the north of the site, via a connection into an existing surface water public sewer that runs through the owner's 'blue-line' boundary. The intention is to re-use the existing outfall to remove the need for a new headwall to be constructed on the watercourse and to avoid any of the adverse ecological impacts associated.

Survey work undertaken at the site has recorded the invert level of the intended connection manhole, as well as levels in the field between the chamber and the red-line boundary. These surveys indicate that a gravity outfall is achievable and that suitable pipe cover can be provided.

Please refer to the updated Drainage Strategy drawing (ref. C3701-DS001 Rev I) for illustration of the revised proposals.

A Section 106 application to connect into the surface water public sewer will be submitted in due course and works will not commence until approval has been granted.

With regards to pollution control/surface water quality, the pollution hazard level for the site is considered to be low (i.e. low traffic roads, residential car parks, driveways).

It is proposed that surface attenuation will be provided within a detention basin lined with an impermeable membrane to prevent any potential transfer of pollutants to groundwater.

All flows emanating from the proposed development are to be routed through the detention basin prior to discharging to watercourse via the public sewer.

This is considered to provide suitable pollution mitigation.

Please refer to Section 14.4 of the Flood Risk Assessment for further commentary on pollution controls and surface water quality.

Proposed Discharge Rate

The submitted hydraulic calculations do not demonstrate that surface water discharge from the site would be restricted as stated in section 14.2 of the above report. Pre-development greenfield runoff has been calculated based on the site area of 1.33 ha and it is proposed to restrict discharge to Q_{bar} 6.8l/s. The total drained area in the post development hydraulic model is 0.569ha which we assume is the total proposed impermeable area of the development. It appears that contribution from permeable areas have been ignored. Due to the discrepancy in drained area used in the greenfield runoff calculation and post development hydraulic calculations, a direct comparison cannot be made. We encourage a catchment-based approach to drainage design in which flows from all contributing areas including both impermeable and permeable surfaces are included in the hydraulic calculations.

It is not clear whether an allowance for urban creep has been included in the hydraulic calculations.

Simulation results for 1 in 1 and 1 in 30 year plus allowance for climate change should be provided.

The proposed discharge rate would be acceptable if the post development calculations were to include all drained areas, including permeable surfaces, to demonstrate that discharge can be restricted to this rate by the proposed drainage system.

As the proposed discharge rate is fundamental to the drainage strategy, we have not concluded a full review of the submitted surface water drainage hydraulic calculations.

A Surface Water Catchment Plan (ref. C3701-DS003) is reproduced within the Flood Risk Assessment to illustrate the permeable and impermeable catchment areas contributing to the proposed drainage network.

Updated greenfield runoff calculations are now based on the total contributing catchment area (0.64ha) and Qbar has been calculated at 3.3 l/s. Refer to Appendix 7 of the updated Flood Risk Assessment.

For the avoidance of doubt, updated hydraulic calculations incorporate:

- i. Impermeable areas with a 10% uplift for urban creep, applied to the receiving node/manhole; and,
- ii. Contributing permeable areas with a 20% impermeability factor applied pro-rata across the network.

An updated drainage strategy plan (ref. C3701-DS001 Rev I) is included in Appendix 7 of the updated Flood Risk Assessment. The amended drawing also indicates the intended location of the proposed connection into the surface water public sewer, and the existing outfall into the Wey Brook to the north of the site.

Exceedance Flow

A post development plan is required to indicate overland flow routes based on the topography / built levels and demonstrating flows will not be directed in new locations and affecting properties. Flows within the development must be routed away from new properties. Details of finished floor levels, road camber and kerb heights may be required.

Exceedance flows will be directed along access roads and around/away from entry points to dwellings. The existing general trend in landform (falling from west to east) will be maintained.

Please refer to the latest Flood Risk Assessment (HSP2021-C3701-C&S-FRAS1-250 Rev I) which includes further commentary relating to the above and an Overland Flow Plan (ref. C3701-DS002) which illustrates flood risk in relation to the proposed layout.

Appendices

Appendix 1 - STL J16117 Southern Testing Factual Ground Investigation Report

**Factual Ground
Investigation Report**

Project Name: Land at Cranesfield

Location: Sherborne St John, Basingstoke, Hampshire

Client: B.Yond Homes Ltd.

Project ID: J16117

Report Date: 25 November 2025

Report Issue: 1

SUMMARY

The site currently comprises part of a larger agricultural field. It is proposed to redevelop the site with housing.

Geological records indicate the site to be underlain by Lambeth Group over Seaford Chalk.

Four cable percussive borehole (BH01-BH04) were carried out to 15m depth for installation of groundwater monitoring wells.

The soils encountered comprised a covering of Topsoil to 0.2/0.5m, over silty/sandy clays to depth, with a layer of dense sand noted between 10.3/14.8m to 11.1/15.0m in all boreholes apart from BH01.

During drilling water strikes were recorded at 5.6m in BH01, 14.8m in BH02, 12.2m in BH03 and 10.3m in BH04.

Digital loggers were placed within each borehole standpipes, on the 14th November 2025, which record water levels at 3 hourly intervals. At the time of writing one monitoring visit has been carried out on 14th November 2025. The water levels recorded to date are 1.27m (76.19AOD) for BH01, 1.18m (77.50m AOD) for BH02, 0.55m (77.53m AOD) for BH03 and 0.64m (78.58m AOD) for BH04.

It should be recognised that site conditions can vary over time, in response to seasonal variations, prevailing weather conditions or other natural phenomena, and also in response to human activity on or near the site. These factors can influence both geotechnical and geoenvironmental characteristics, including soil moisture conditions, the stability of the ground, the occurrence of groundwater, and the behaviour of mobile contamination phases. Accordingly, the accuracy and reliability of the investigation findings may be affected.

The investigation was conducted and this report has been prepared for the sole internal use and reliance of B.Yond Homes Ltd. and their appointed Engineers. This report shall not be relied upon or transferred to any other parties without the express written authorisation of Southern Testing Laboratories Ltd. If an unauthorised third party comes into possession of this report they rely on it at their peril and the authors owe them no duty of care and skill.

The findings and opinions conveyed via this investigation report are based on information obtained from a variety of sources as detailed within this report, and which Southern Testing Laboratories Ltd. believes are reliable. Nevertheless, Southern Testing Laboratories Ltd. cannot and does not guarantee the authenticity or reliability of the information it has obtained from others.

Southern Testing Laboratories Ltd. assumes no liability for any conditions not revealed in the records obtained in performance of the services in accordance with the limitations of the contract with the Client, or where variations occur between exploratory hole locations or below the depth of investigation.

The recommendations contained in this report are made in respect of the particular context of the investigation as described in the report and may not be appropriate to alternative development schemes. This report should be considered in its entirety and Southern Testing Laboratories Ltd. accepts no responsibility for and excludes liability in respect of any omission or alteration made by others, and any use of the report for any purpose other than that for which it was produced.

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For and on behalf of Southern Testing Laboratories Limited

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APPENDIX B

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A INTRODUCTION

1 Authority

Our authority for carrying out this work is contained in a signed deed of appointment from B.Yond Homes Ltd. dated 24th November 2025. This is in relation to our quotation (Q251281) dated 17th October 2025.

2 Location

The site is located north-east of Cranesfield Road in Sherborne St John. The approximate National Grid Reference of the site is SU 62250 55689. The site location is indicated on Figure 1 within Appendix A.

3 Proposed Construction

It is proposed to develop the site with housing. Southern Testing have not been provided with proposed development plans.

4 Object

This is a factual ground investigation report.

The object of the investigation was to install groundwater monitoring wells for winter groundwater monitoring and consider the potential for shallow soakaway drainage.

5 Scope

This factual report presents our exploratory hole logs and test results only.

A UXO risk assessment was not requested within our brief for the investigation.

As with any site there may be differences in soil conditions between exploratory hole positions and below the depth of investigation. Southern Testing assumes no liability in respect of such differences.

This report is not an engineering design and the figures and calculations contained in the report should be used by the Engineer, taking note that variations will apply, according to variations in design loading, in techniques used, and in site conditions. Our figures therefore should not supersede the Engineer's design.

The ground investigation has been completed with reference to BS 5930 Ref [1] and BS 10175 Ref [2].

Contamination & Geotechnical issues are not considered in this report.

It should be recognised that site conditions can vary over time, in response to seasonal variations, prevailing weather conditions or other natural phenomena, and also in response to human activity on or near the site. These factors can influence both geotechnical and geoenvironmental characteristics, including soil moisture conditions, the stability of the ground, the occurrence of groundwater, and the behaviour of mobile contamination phases. Accordingly, the accuracy and reliability of the investigation findings may be affected. The findings and opinions conveyed via this investigation report are based on information obtained from a variety of sources as detailed within this report, and which Southern Testing Laboratories Ltd. believes are reliable. Nevertheless, Southern Testing Laboratories Ltd. cannot and does not guarantee the authenticity or reliability of the information it has obtained from others.

The investigation was conducted and this report has been prepared for the sole internal use and reliance of B.Yond Homes Ltd. and their appointed Engineers. This report shall not be relied upon or transferred to any other parties without the express written authorisation of Southern Testing Laboratories Ltd. If an unauthorised third party comes into possession of this report they rely on it at their peril and the authors owe them no duty of care and skill.

The recommendations contained in this report are made in respect of the particular context of the investigation as described in the report and may not be appropriate to alternative development schemes. This report should be considered in its entirety and Southern Testing Laboratories Ltd. accepts no responsibility for and excludes liability in respect of any omission or alteration made by others, and any use of the report for any purpose other than that for which it was produced.

Detailed information on the proposed development, such as detailed final layout, loadings and serviceability limits was not provided. Accordingly, where geotechnical design advice is provided it is on the prescriptive basis allowed for by Eurocode 7: employing conventional and conservative design rules.

B BACKGROUND INFORMATION

6 Geology

The British Geological Survey Map No. 284 (Basingstoke) indicates that the site geology consists of Lambeth Group over Seaford Chalk.

6.1 Lambeth Group

The Lambeth Group comprises a vertically and laterally very variable sequence of multicoloured and mottled clays and sands. The sands are greenish yellow or brown, and generally alternate with the multicoloured mottled clays and sometimes bands of lignite. Pebble Beds, locally cemented into conglomerate, and some bands of concretionary ironstone may also be present. Shells are frequently found in the clays and are sometimes locally cemented into a limestone bed that may form an obstruction to pile borings. At the base of the formation there can be layer of greenish sands with flint pebbles where they rest directly above the Thanet Beds.

Clays within this group are known to contain pyrite.

6.2 Seaford Chalk Formation

The Seaford Chalk Formation comprises a fairly homogeneous white chalk with regular and conspicuous flint bands. These flints are commonly laterally very continuous and traceable over large distances. Some of the flints can be very large. A few marl seams are present within the lower parts of the formation.

The White Chalk outcrop in particular is frequently highly fractured and highly permeable, and usually has good infiltration characteristics. On the other hand, Chalk Head, highly weathered Chalk and Chalk under a low permeability superficial cover may have very poor infiltration characteristics.

Chalk is slightly soluble in water and, while it has excellent bearing properties when unweathered, this solubility can lead to deep weathering and softening, and the upper layers of chalk often have an irregular boundary with overlying strata

The Chalk may be softened by solution to a depth of 5 to 15 metres and bearing capacities and engineering properties improve with depth. Where there is an outcrop of impermeable soil overlying the chalk there may be a dramatically increased solution effect due to concentrated surface water flow to the Chalk close to the outcrop boundary.

Solution features are common in the Chalk, and these can present significant difficulties to development on affected sites.

Man has also worked the chalk for flints, and for other purposes, for thousands of years and any signs of old workings should be carefully investigated.

7 Site Description

The site is currently the southern part of an agricultural field. The west, south and eastern boundaries are defined by fencing, hedging and trees. There is no defined northern boundary.

To the west and south-west of the site is residential housing and to the east are larger residential properties with associated gardens which bound the site. To the south of the site is a cemetery, and adjacent to the south-eastern boundary is a pond.

7.1 Site Photographs

A series of photographs showing the site is included in Appendix B.

C GROUND INVESTIGATION

8 Strategy and Method

The strategy adopted for the intrusive investigation comprised the following:

Activity / Method	Purpose	Max Depth Range (mbgl)	Installations / Notes
BH01-BH04 Cable Percussive	Boreholes for installation of groundwater monitoring wells.	15m	150mm diameter

Exploratory hole locations were specified by the Clients Engineer and are shown in Figure 2 in Appendix A.

In-situ test and sampling methods descriptions employed are given in Appendix B together with the test results.

9 Weather Conditions

The cable percussive boreholes were carried out between the 6th and 11th November 2025 at which time the weather was generally overcast with rain.

From Met Office data, the rainfall in the south of England was 42% the normal average for August, 129% the normal average for September and 105% the normal average for October.

10 Soils as Found

The soils encountered are described in detail in the attached exploratory hole logs (Appendix A), but in general comprised a covering of Topsoil over Lambeth Group soils. A summary is given below.

Depth (m)	Thickness (m)	Soil Type	Description
GL to 0.2/0.5m	0.2m to 0.5m	Topsoil	Dark brown, silty CLAY with small gravel and roots. (Drillers Description)
0.2/0.5m to 1.7/6.3m	1.2m to 6.1m	Lambeth Group	Stiff to very stiff, sandy CLAY, occasionally gravelly. (Drillers Description)
1.7/6.3m to 15.0m+	8.7m		Very stiff, CLAY/silty CLAY. (Drillers Description)
10.3/14.8m to 11.1/15.0m	0.2m to 2.4m		Dense SAND. (Drillers Description) Not encountered in BH01.
14.6m to 15.0m in BH03 11.1m to 12.3m in BH04	0.4m to 1.2m		Very stiff, sandy CLAY/CLAY with sand bands. (Drillers Description) Not encountered in BH01 or BH02

10.1 Visual and Olfactory Evidence of Contamination

No visual or olfactory evidence of contamination was identified during excavation of the boreholes.

11 Groundwater Observations

Groundwater was observed in the exploratory holes as follows:

Hole ID	Water Strike Depth (m)	Comment	Stratum
BH01	5.6m	Water rose to 5.4m after 20 minutes.	Lambeth Group
BH02	14.8m	Water rose to 5.9m after 20 minutes.	Lambeth Group
BH03	12.2m	Water rose to 7.9m after 20 minutes.	Lambeth Group
BH04	10.3m	Water rose to 6.7m after 20 minutes.	Lambeth Group

Groundwater levels vary considerably from season to season and year to year, often rising close to the ground surface in wet or winter weather, and falling in periods of drought. Groundwater levels measured in wet or Winter months may be significantly higher than those that might be experienced in dry or Summer months and the possibility of a requirement for dewatering or alternative foundation solution that may not have initially been considered should not be overlooked.

The effects of climate change, particularly the increased likelihood of significant rainfall events and warmer, wetter winters may mean groundwater levels in the future could be higher than those measured during site investigation works.

12 Groundwater Monitoring

Each borehole was installed with a groundwater monitoring well. Monitoring well installation details are described on the borehole logs in Appendix A.

The groundwater monitoring wells consists of a perforated pipe, which is installed in the ground. The standpipe is 50mm in diameter. Each monitoring well is perforated from the base with a sand/gravel surround through the Lambeth Group to 1m below ground level. Above this there is a bentonite seal with solid pipework and is provided with a bung at the top. Each well was completed at the surface with a raised cover.

Digital loggers were placed within each borehole standpipe, on the 14th November 2025, which record water levels at 3 hourly intervals. A monitoring visit to manually dip the water level, using a dip meter, and to download the digital loggers data will be carried out on a monthly basis. An updated report will be issued on completion of the groundwater monitoring.

Borehole Id	Recorded Groundwater Level (14/11/2025)	
	mbgl	mAOD
BH01	1.27m	76.19m
BH02	1.18m	77.50m
BH03	0.55m	77.53m
BH04	0.64m	78.58m

On completion of the monitoring the existing monitoring wells on the site should be decommissioned, to prevent the downward migration of contamination to the groundwater.

13 Soakaways

No soakage testing has been carried out, however, due to the clay nature of the soils encountered on site these are considered unsuitable for conventional soakaway drainage and a positive drainage system or potentially deep borehole soakaways should be considered for surface water disposal.

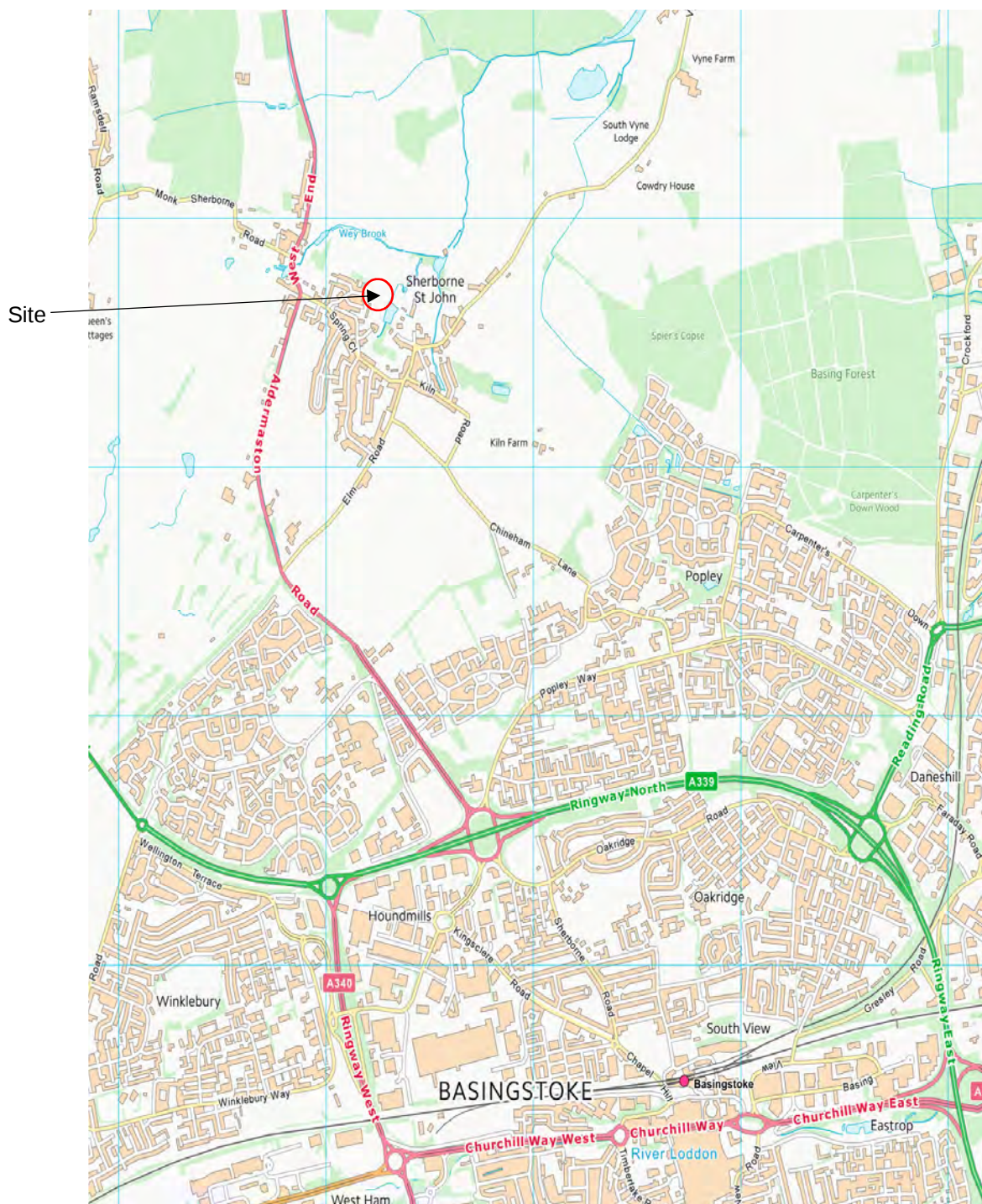
REFERENCES

- [1] BSI Standards, "BS 5930 Code of practice for ground investigations," 2015.
- [2] BSI Standards, "BS10175 Investigation of potentially contaminated sites – Code of practice," 2013.
- [3] BSI Standards, "BS 3882:2015 Specification for Topsoil," 2015.

APPENDIX A

**Site Plans and Exploratory
Hole Logs**



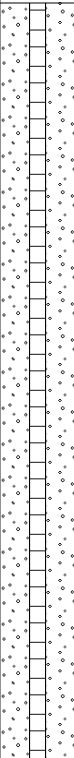


Contains Ordnance Survey Data © Crown Copyright and Database Right 2025

Site:	Land at Cranesfield, Sherborne St John	Project ID	J16117
Figure 1	Site Location Plan	Date:	25/11/2025



Project Name:	Land at Cranesfield	Project ID:	J16117	Figure 2: Borehole Location Plan  Southern Testing Environmental & Geotechnical
Location:	Sherborne St. John, Basingstoke	Engineer:	VF	
Client:	B.Yond Homes Ltd.	Scale:	1:1000	

				Start - End Date:		Project ID:		Hole Type:		BH01	
				10/11/2025		J16117		CP		Sheet 2 of 2	
Client:		B.Yond Homes Ltd.				Co-ordinates:		Level (m AOD)		Logger:	
						E 462320 - N 155736		77.46		RF	
Project Name:		Land at Cranesfield				Location:		Sherborne St. John, Basingstoke			
Well	Water Strikes	Samples and Insitu Testing			Level (m AOD)	Thickness (m)	Legend	Depth (m bgl)	Stratum Description		
		Depth (m bgl)	Type	Results							
					(4.80)		14.70 14.90 15.00	Very stiff, reddish brown and grey, CLAY. (Drillers Description)			
				62.76 62.56 62.46	(0.20) (0.10)			Very stiff, bluish grey, silty CLAY. (Drillers Description) Very stiff, reddish brown and grey, CLAY. (Drillers Description) End of Borehole at 15.00m			
Remarks:											
Borehole for installation of groundwater monitoring well. Monitoring well installed with response zone between 1m and 15mbgl.											
Status:		FINAL		Log Print Date and Time:		24/11/2025 09:42		Log Approved By:		VF	

	Start - End Date:	Project ID:	Hole Type:	BH01
	10/11/2025	J16117	CP	Sheet 1 of 1

Depth Related Hole Information									
Top Depth (m)	Base Depth (m)	Hole Type	Start Date	End Date	Crew	Plant Used	Contractor	Logged By	Remarks
0.00 1.20	1.20 15.00	IP CP	10/11/2025 10/11/2025	10/11/2025 10/11/2025	RF RF	Hand Dug Dando	STL STL	RF RF	

Hole Diameter			Casing Diameter		
Hole Diameter (mm)	Depth to Base (m)	Remarks	Casing Diameter (mm)	Depth to Base (m)	Remarks
150	15.00		150	12.00	

Location Specific Time Related Remarks				Drilling Progress by Time				
Date Time Start	Date Time Start	Duration	Remarks	Date Time	Depth (m)	Casing Depth (m)	Water Depth (m)	Remarks
				10/11/2025 10:00:00 10/11/2025 16:00:00	15.00	6.00	12	Start of borehole. End of drilling.

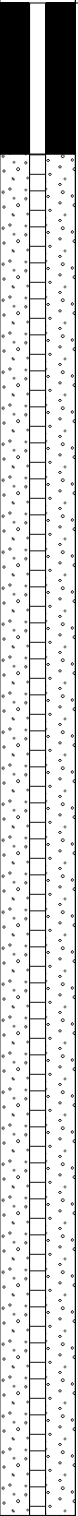
SPT								
Top Depth (m)	Test Type	Casing Depth (m)	Water Depth (m)	Self Weight Penetration (mm)	Reported Result (N Value)	Hammer Serial No.	Energy Ratio %	Remarks

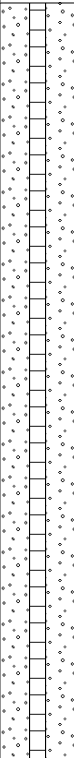
Water Strikes and Details							
Depth Strike (m)	Date Time	Depth Casing (m)	Depth Sealed (m)	Remarks	Time Elapsed (mins)	Water Depth (m)	Remarks
5.60	10/11/2025 00:00:00	5.60			20	5.40	

Backfill Details				Chiselling Details				
Depth Top (m)	Depth Base (m)	Material	Remarks	From (m)	To (m)	Start (Date / Time)	Duration (hh:mm)	Remarks
0.00 1.00	1.00 15.00	Bentonite Gravel Backfill						

Installation Details						Pipe Construction				
Distance	Point Ref.	Type	Top of Response Zone (m)	Base of Response Zone (m)	Pipe Ref.	Pipe Reference	Depth Top (m)	Depth Base (m)	Diameter (mm)	Pipe Type
15.00	BH01	SP	1.00	15.00	BH01	BH01 BH01	0.00 1.00	1.00 15.00	50 50	PLAIN SLOTTED

Status:	FINAL	Log Print Date and Time:	24/11/2025 09:42	Log Approved By:	VF
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				Start - End Date:		Project ID:	Hole Type:	BH02		
				11/11/2025		J16117	CP	Sheet 1 of 2		
Client:		B.Yond Homes Ltd.				Co-ordinates:		Level (m AOD)	Logger:	
						E 462259 - N 155732		78.68	RF	
Project Name:		Land at Cranesfield				Location:		Sherborne St. John, Basingstoke		
Well	Water Strikes	Samples and Insitu Testing			Level (m AOD)	Thickness (m)	Legend	Depth (m bgl)	Stratum Description	
		Depth (m bgl)	Type	Results						
					78.48	(0.20)		0.20	Dark brown, silty CLAY with rare small gravel and small roots. TOPSOIL (Drillers Description)	
						(0.90)			Stiff, brown, gravelly CLAY. (Drillers Description)	1
					77.58			1.10	Stiff brown CLAY with rare gravel. (Drillers Description)	
					76.98	(0.60)		1.70	Stiff, brown, sandy gravelly CLAY. (Drillers Description)	2
						(1.30)				
					75.68	(0.20)		3.00	Brown, sandy CLAY. (Drillers Description)	3
					75.48	(0.60)		3.20	Stiff, grey, silty sandy CLAY. (Drillers Description)	
					74.88			3.80	Very stiff, brown, CLAY with rare chalk. (Drillers Description)	4
						(3.60)				5
										6
								7		
								8		
								9		
								10		
Remarks:										
Borehole for installation of groundwater monitoring well. Monitoring well installed with response zone between 1m and 15mbgl.										
Status:		FINAL		Log Print Date and Time:		24/11/2025 09:42		Log Approved By:		VF

				Start - End Date:		Project ID:	Hole Type:	BH02		
				11/11/2025		J16117	CP	Sheet 2 of 2		
Client:		B.Yond Homes Ltd.				Co-ordinates:		Level (m AOD)	Logger:	
						E 462259 - N 155732		78.68	RF	
Project Name:		Land at Cranesfield				Location:	Sherborne St. John, Basingstoke			
Well	Water Strikes	Samples and Insitu Testing			Level (m AOD)	Thickness (m)	Legend	Depth (m bgl)	Stratum Description	
		Depth (m bgl)	Type	Results						
						(7.40)			Very stiff, reddish brown and grey, CLAY. (Drillers Description)	11
										12
										13
										14
					63.88			14.80		
					63.68	(0.20)		15.00	Dense, buff, SAND.	15
									End of Borehole at 15.00m	
										16
										17
										18
										19
										20
Remarks:										
Borehole for installation of groundwater monitoring well. Monitoring well installed with response zone between 1m and 15mbgl.										
Status:		FINAL		Log Print Date and Time:		24/11/2025 09:42		Log Approved By:		VF

	Start - End Date:	Project ID:	Hole Type:	BH02
	11/11/2025	J16117	CP	Sheet 1 of 1

Depth Related Hole Information									
Top Depth (m)	Base Depth (m)	Hole Type	Start Date	End Date	Crew	Plant Used	Contractor	Logged By	Remarks
0.00 1.20	1.20 15.00	IP CP	11/11/2025 11/11/2025	11/11/2025 11/11/2025	RF RF	Hand Dug Dando	STL STL	RF RF	

Hole Diameter			Casing Diameter		
Hole Diameter (mm)	Depth to Base (m)	Remarks	Casing Diameter (mm)	Depth to Base (m)	Remarks
150	15.00		150	12.00	

Location Specific Time Related Remarks				Drilling Progress by Time				
Date Time Start	Date Time Start	Duration	Remarks	Date Time	Depth (m)	Casing Depth (m)	Water Depth (m)	Remarks
				11/11/2025 08:00:00 11/11/2025 16:00:00	15.00	12.00	5	Start of borehole. End of drilling.

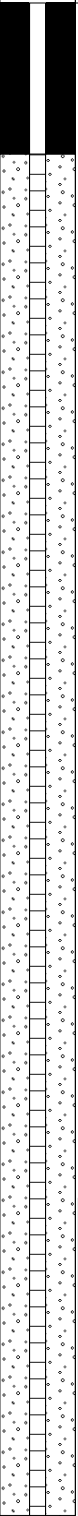
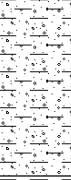
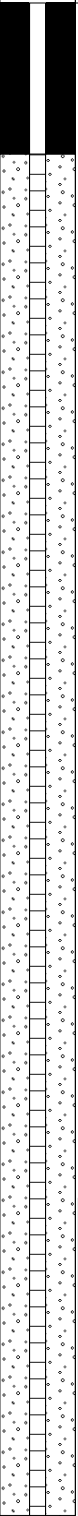
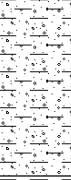
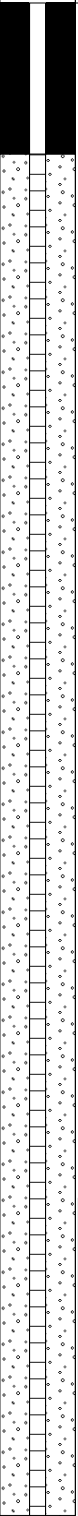
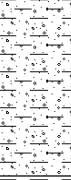
SPT								
Top Depth (m)	Test Type	Casing Depth (m)	Water Depth (m)	Self Weight Penetration (mm)	Reported Result (N Value)	Hammer Serial No.	Energy Ratio %	Remarks

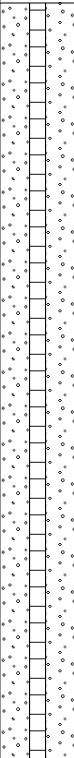
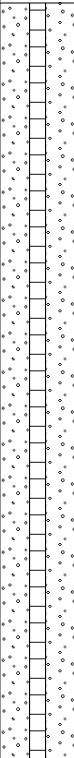
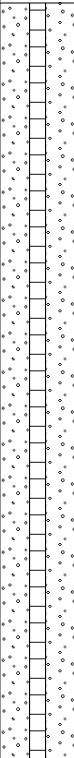
Water Strikes and Details							
Depth Strike (m)	Date Time	Depth Casing (m)	Depth Sealed (m)	Remarks	Time Elapsed (mins)	Water Depth (m)	Remarks
14.80	11/11/2025 00:00:00	12.00			20	5.90	

Backfill Details				Chiselling Details				
Depth Top (m)	Depth Base (m)	Material	Remarks	From (m)	To (m)	Start (Date / Time)	Duration (hh:mm)	Remarks
0.00 1.00	1.00 15.00	Bentonite Gravel Backfill						

Installation Details						Pipe Construction				
Distance	Point Ref.	Type	Top of Response Zone (m)	Base of Response Zone (m)	Pipe Ref.	Pipe Reference	Depth Top (m)	Depth Base (m)	Diameter (mm)	Pipe Type
15.00	BH02	SP	1.00	15.00	BH02	BH02 BH02	0.00 1.00	1.00 15.00	50 50	PLAIN SLOTTED

Status:	FINAL	Log Print Date and Time:	24/11/2025 09:42	Log Approved By:	VF
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				Start - End Date:		Project ID:	Hole Type:	BH03																																																																		
				07/11/2025 - 10/11/2025		J16117	CP	Sheet 1 of 2																																																																		
Client:		B.Yond Homes Ltd.				Co-ordinates:		Level (m AOD)	Logger:																																																																	
						E 462279 - N 155669		78.08	RF																																																																	
Project Name:		Land at Cranesfield				Location:	Sherborne St. John, Basingstoke																																																																			
<table><tr><td rowspan="2">Well</td><td rowspan="2">Water Strikes</td><td colspan="3">Samples and Insitu Testing</td><td rowspan="2">Level (m AOD)</td><td rowspan="2">Thickness (m)</td><td rowspan="2">Legend</td><td rowspan="2">Depth (m bgl)</td><td rowspan="2">Stratum Description</td><td rowspan="2"></td></tr><tr><td>Depth (m bgl)</td><td>Type</td><td>Results</td></tr><tr><td rowspan="5"></td><td></td><td></td><td></td><td></td><td>77.58</td><td>(0.50)</td><td></td><td>0.50</td><td>Dark brown, silty CLAY with rare gravel and small roots. TOPSOIL (Drillers Description)</td><td>1</td></tr><tr><td></td><td></td><td></td><td></td><td>76.38</td><td>(1.20)</td><td></td><td>1.70</td><td>Stiff, grey, sandy gravelly CLAY. (Drillers Description)</td><td>2</td></tr><tr><td></td><td></td><td></td><td></td><td></td><td>(2.40)</td><td></td><td>4.10</td><td>Stiff, brownish grey, CLAY. (Drillers Description)</td><td>3</td></tr><tr><td></td><td></td><td></td><td></td><td>73.98</td><td>(5.70)</td><td></td><td>9.80</td><td>Very stiff, reddish brown and grey, CLAY. (Drillers Description)</td><td>4</td></tr><tr><td></td><td></td><td></td><td></td><td>68.28</td><td>(0.30)</td><td></td><td></td><td>Very stiff, bluish grey, silty CLAY. (Drillers Description)</td><td>5</td></tr></table>										Well	Water Strikes	Samples and Insitu Testing			Level (m AOD)	Thickness (m)	Legend	Depth (m bgl)	Stratum Description		Depth (m bgl)	Type	Results						77.58	(0.50)		0.50	Dark brown, silty CLAY with rare gravel and small roots. TOPSOIL (Drillers Description)	1					76.38	(1.20)		1.70	Stiff, grey, sandy gravelly CLAY. (Drillers Description)	2						(2.40)		4.10	Stiff, brownish grey, CLAY. (Drillers Description)	3					73.98	(5.70)		9.80	Very stiff, reddish brown and grey, CLAY. (Drillers Description)	4					68.28	(0.30)			Very stiff, bluish grey, silty CLAY. (Drillers Description)	5
Well	Water Strikes	Samples and Insitu Testing			Level (m AOD)	Thickness (m)	Legend	Depth (m bgl)	Stratum Description																																																																	
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Status:		FINAL		Log Print Date and Time:		24/11/2025 09:42		Log Approved By:		VF																																																																

				Start - End Date:		Project ID:	Hole Type:	BH03																																																																																																													
				07/11/2025 - 10/11/2025		J16117	CP	Sheet 2 of 2																																																																																																													
Client:		B.Yond Homes Ltd.				Co-ordinates:		Level (m AOD)	Logger:																																																																																																												
						E 462279 - N 155669		78.08	RF																																																																																																												
Project Name:		Land at Cranesfield				Location:	Sherborne St. John, Basingstoke																																																																																																														
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Well	Water Strikes	Samples and Insitu Testing			Level (m AOD)	Thickness (m)	Legend	Depth (m bgl)	Stratum Description																																																																																																												
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					65.88	(2.40)		12.20	Dense, buff, SAND. (Drillers Description)	13																																																																																																											
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			63.08					15.00	End of Borehole at 15.00m	16																																																																																																											
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Borehole for installation of groundwater monitoring well. Monitoring well installed with response zone between 1m and 15mbgl.																																																																																																																					
Status:		FINAL		Log Print Date and Time:		24/11/2025 09:42		Log Approved By: VF																																																																																																													

	Start - End Date:	Project ID:	Hole Type:	BH03
	07/11/2025 - 10/11/2025	J16117	CP	Sheet 1 of 1

Depth Related Hole Information									
Top Depth (m)	Base Depth (m)	Hole Type	Start Date	End Date	Crew	Plant Used	Contractor	Logged By	Remarks
0.00 1.20	1.20 15.00	IP CP	07/11/2025 07/11/2025	07/11/2025 10/11/2025	RF RF	Hand Dug Dando	STL STL	RF RF	

Hole Diameter			Casing Diameter		
Hole Diameter (mm)	Depth to Base (m)	Remarks	Casing Diameter (mm)	Depth to Base (m)	Remarks
150	15.00		150	15.00	

Location Specific Time Related Remarks				Drilling Progress by Time				
Date Time Start	Date Time Start	Duration	Remarks	Date Time	Depth (m)	Casing Depth (m)	Water Depth (m)	Remarks
				07/11/2025 13:00:00				Start of borehole.
				07/11/2025 16:00:00	13.50	13.50	7.9	End of shift.
				10/11/2025 08:00:00	13.50	13.50	1.2	Start of stift.
				10/11/2025 09:00:00	15.00	15.00	13	End of drilling.

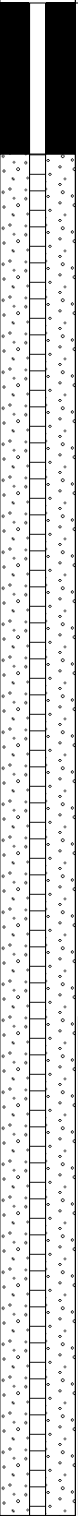
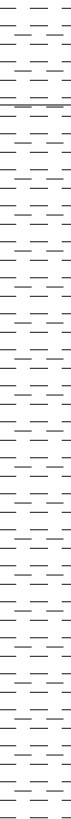
SPT								
Top Depth (m)	Test Type	Casing Depth (m)	Water Depth (m)	Self Weight Penetration (mm)	Reported Result (N Value)	Hammer Serial No.	Energy Ratio %	Remarks

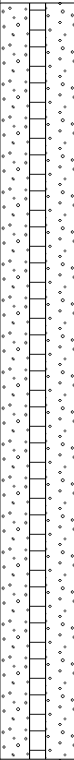
Water Strikes and Details							
Depth Strike (m)	Date Time	Depth Casing (m)	Depth Sealed (m)	Remarks	Time Elapsed (mins)	Water Depth (m)	Remarks
12.20	07/11/2025 00:00:00	12.00			20	7.90	

Backfill Details				Chiselling Details				
Depth Top (m)	Depth Base (m)	Material	Remarks	From (m)	To (m)	Start (Date / Time)	Duration (hh:mm)	Remarks
0.00 1.00	1.00 15.00	Bentonite Gravel Backfill						

Installation Details						Pipe Construction				
Distance	Point Ref.	Type	Top of Response Zone (m)	Base of Response Zone (m)	Pipe Ref.	Pipe Reference	Depth Top (m)	Depth Base (m)	Diameter (mm)	Pipe Type
15.00	BH03	SP	1.00	15.00	BH03	BH03 BH03	0.00 1.00	1.00 15.00	50 50	PLAIN SLOTTED

Status:	FINAL	Log Print Date and Time:	24/11/2025 09:42	Log Approved By:	VF
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				Start - End Date:		Project ID:	Hole Type:	BH04			
				06/11/2025 - 07/11/2025		J16117	CP	Sheet 1 of 2			
Client:		B.Yond Homes Ltd.				Co-ordinates:		Level (m AOD)	Logger:		
						E 462228 - N 155691		79.22	RF		
Project Name:		Land at Cranesfield				Location:		Sherborne St. John, Basingstoke			
Well	Water Strikes	Samples and Insitu Testing			Level (m AOD)	Thickness (m)	Legend	Depth (m bgl)	Stratum Description		
		Depth (m bgl)	Type	Results							
					78.72	(0.50)		0.50	Dark brown silty CLAY with rare small gravel and small roots. (Drillers Description) TOPSOIL		
					77.92	(0.80)		1.30	Stiff, grey, sandy gravelly CLAY. (Drillers Description)	1	
					77.52	(0.40)		1.70	Stiff, brown, CLAY with occasional gravel. (Drillers Description)	2	
					76.02	(1.50)		3.20	Very stiff, brown, CLAY. (Drillers Description)	3	
						(4.80)			Very stiff, reddish brown and grey, CLAY. (Drillers Description)	4	
										5	
					71.22	(0.40)		8.00	Very stiff, bluish grey, silty CLAY. (Drillers Description)	8	
					70.82	(1.90)		8.40	Very stiff, reddish brown and grey, CLAY. (Drillers Description)	9	
											10
		Remarks:									
Borehole for installation of groundwater monitoring well. Monitoring well installed with response zone between 1m and 15mbgl.											
Status:		FINAL		Log Print Date and Time:		24/11/2025 09:42		Log Approved By:		VF	

				Start - End Date:		Project ID:	Hole Type:	BH04		
				06/11/2025 - 07/11/2025		J16117	CP	Sheet 2 of 2		
Client:		B.Yond Homes Ltd.				Co-ordinates:		Level (m AOD)	Logger:	
						E 462228 - N 155691		79.22	RF	
Project Name:		Land at Cranesfield				Location:		Sherborne St. John, Basingstoke		
Well	Water Strikes	Samples and Insitu Testing			Level (m AOD)	Thickness (m)	Legend	Depth (m bgl)	Stratum Description	
		Depth (m bgl)	Type	Results						
					68.92	(0.80)		10.30	Very stiff, reddish brown and grey, CLAY. (Drillers Description)	11
					68.12			11.10	Dense, buff, SAND. (Drillers Description)	
						(1.20)		12.30	Very stiff, brown, CLAY with buff sand bands. (Drillers Description)	12
					66.92			13	Very stiff, brownish grey, silty CLAY. (Drillers Description)	
						(2.70)		15.00		End of Borehole at 15.00m
			64.22							16
									17	
									18	
									19	
									20	
Remarks:										
Borehole for installation of groundwater monitoring well. Monitoring well installed with response zone between 1m and 15mbgl.										
Status:		FINAL		Log Print Date and Time:		24/11/2025 09:42		Log Approved By:		VF

	Start - End Date:	Project ID:	Hole Type:	BH04
	06/11/2025 - 07/11/2025	J16117	CP	Sheet 1 of 1

Depth Related Hole Information									
Top Depth (m)	Base Depth (m)	Hole Type	Start Date	End Date	Crew	Plant Used	Contractor	Logged By	Remarks
0.00 1.20	1.20 15.00	IP CP	06/11/2025 06/11/2025	06/11/2025 07/11/2025	RF RF	Hand Dug Dando	STL STL	RF RF	

Hole Diameter			Casing Diameter		
Hole Diameter (mm)	Depth to Base (m)	Remarks	Casing Diameter (mm)	Depth to Base (m)	Remarks
150	15.00		150	13.50	

Location Specific Time Related Remarks				Drilling Progress by Time				
Date Time Start	Date Time Start	Duration	Remarks	Date Time	Depth (m)	Casing Depth (m)	Water Depth (m)	Remarks
				06/11/2025 11:00:00				Start of borehole.
				06/11/2025 16:00:00	12.00	12.00	6.5	End of shift.
				07/11/2025 08:00:00	12.00	12.00	3.3	Start of shift.
				07/11/2025 11:00:00	15.00	13.50	14.8	End of drilling.

SPT								
Top Depth (m)	Test Type	Casing Depth (m)	Water Depth (m)	Self Weight Penetration (mm)	Reported Result (N Value)	Hammer Serial No.	Energy Ratio %	Remarks

Water Strikes and Details							
Depth Strike (m)	Date Time	Depth Casing (m)	Depth Sealed (m)	Remarks	Time Elapsed (mins)	Water Depth (m)	Remarks
10.30	06/11/2025 00:00:00	10.00			20	6.70	

Backfill Details				Chiselling Details				
Depth Top (m)	Depth Base (m)	Material	Remarks	From (m)	To (m)	Start (Date / Time)	Duration (hh:mm)	Remarks
0.00 1.00	1.00 15.00	Bentonite Gravel Backfill						

Installation Details						Pipe Construction				
Distance	Point Ref.	Type	Top of Response Zone (m)	Base of Response Zone (m)	Pipe Ref.	Pipe Reference	Depth Top (m)	Depth Base (m)	Diameter (mm)	Pipe Type
15.00	BH04	SP	1.00	15.00	BH04	BH04 BH04	0.00 1.00	1.00 15.00	50 50	PLAIN SLOTTED

Status:	FINAL	Log Print Date and Time:	24/11/2025 09:42	Log Approved By:	VF
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Key to Exploratory Hole Logs, Plans and Sections

Backfill Symbols		Pipe Symbols		Principal Soil Types		Principal Rock Types		Drilling Records	
Arisings		Plain Pipe		Topsoil		Mudstone		Water Strike	
Concrete		Slotted Pipe		Made Ground		Claystone		Depth Water Rose	
Blacktop		Piezometer		Clay		Siltstone		Total Core Recovery (%) [TCR]	
Bentonite		Piezometer Tip		Silt		Sandstone		Solid Core Recovery (%) [SCR]	
Gravel Filter		Filter Tip		Sand		Limestone		Rock Quality Index (%) RQD]	
Sand Filter		Extensometer		Gravel		Chalk		Fracture Index (fractures / m) [FI]	
		Inclinometers		Peat					

All soil and rock descriptions are in general accordance with BS5930 2015, BS EN ISO 14688-1:2002+A1:2013 and BS EN ISO 14689-1:2003. Chalk descriptions are also based on CIRIA C574 and "Logging the Chalk – R.N. Mortimer 2015". The Geology Code is only provided where a positive identification of the sample strata has been made.

Location / Method Identifiers		In-situ Test Location / Method	
BH	Borehole (undefined)	DP	Dynamic Probe
CP	Cable Percussive	CPT	Cone Penetration Test
RC	Rotary Core	CBR	In-situ CBR Test
RO	Rotary Open Hole	DCP	CBR using Dynamic Cone Penetrometer
ODC	Rotary Odex/Symmetrix drilling cased	CBRT	CBR using TRL Probe
CP+RC	Cable Percussive to Rotary Core	PB	Plate Bearing Test
SNC	Sonic	SPT (S)	Standard Penetration Test (Split Barrel Sampler)
CFA	Continuous Flight Auger	SPT (C)	Standard Penetration Test (Solid Cone)
FA	Flight Auger	N	SPT Result
VC	Vibro Core	-/-	Blows/Penetration (mm) after seating drive
WLS+RC	Windowless (Dynamic) Sampler to Rotary Core	-*/-	Total Blows / Penetration (mm)
WLS	Windowless Sampler	()	Extrapolated Value
WS	Window Sampler	PPT	Perth Penetration (In-House Method - Equivalent N Value)
HA	Hand Auger	HP / UCS	Strength from Hand Penetrometer (kN/m ²)
C	Road / Pavement Core	IVN	Strength from Hand Vane ((kN/m ²) P = peak, R = residual
IP	Inspection Pit (Hand Excavation)	PID	Photo Ionisation Detector (ppm)
TP	Trial Pit (Machine Excavated)	MEXE	Mexi-Cone CBR (%)
OP	Observation Pit (Supported Excavation Hand or Machine)		

Samples / Test Type	
B	Bulk Sample
BLK	Block Sample
C	Core Sample
CBRS	CBR Mould Sample
D	Small Disturbed Sample
ES	Environmental Sample (Soil)
EW	Environmental Sample (Water)
GS	Environmental Sample (Gas)

Samples / Test Type	
SPTLS	Standard Penetration Test Split Barrel Sample
TW	Thin Wall Push In Sample (e.g. Shelby Sampler)
U	Undisturbed Open Drive Sample (blows to take)
UT	Thin Wall Undisturbed Open Drive Sample (blows to take)
W	Water Sample (Geotechnical)
SP	Sample from Stockpile
P	Piston Sample
AMAL	Amalgamated Sample

Soil and Rock Descriptions

All soil and rock descriptions are in general accordance with BS5930 Ref [1].

Anthropogenic soils ('made ground' or 'fill') describe materials which have been placed by man and can be divided into those composed of reworked natural soils and those composed of or containing man-made materials. 'Fill' is used to describe material placed in a controlled manner and 'made ground' is used to describe materials placed without strict engineering control.

The classification of materials such as topsoil is based on visual description only and should not be interpreted to mean that the material complies with criteria used in BS 3882 Ref [3].

The geology code is only provided on logs where a positive identification of the sample strata has been made.

Inspection Pit

Inspection pits are hand excavated from the surface (maximum depth 1.2 – 1.5m) using appropriate tools to locate and avoid existing buried services at exploratory hole positions. They are also regularly used as part of investigations on existing structures to expose and determine foundation detail.

Monitoring Well

A groundwater and/or ground gas monitoring well consists of a perforated pipe, which is installed in the ground. The standpipe is typically 50mm nominal in diameter and is installed in a lined borehole. It is perforated from the base with a sand/gravel surround through the soil horizon of interest to an appropriate depth below ground level. Above this there is a bentonite seal with solid pipework and is provided with an end cap or a gas valve at the top as appropriate.

Gas monitoring is carried out via the gas tap. Water sampling/purging can be undertaken by removing the gas tap and bung.

The well is usually completed at the surface with a flush cast iron cover or raised lockable cover.

Groundwater Monitoring – Dip Meter

The dip meter is used to measure standing water levels within boreholes. The probe is lowered into the borehole until the meter detects the groundwater with an audible 'beep'. The level is then read from the tape.

Groundwater Monitoring – Digital Level Loggers

Digital loggers placed within borehole standpipes measuring water pressure, barometric pressure and temperature were used to record data over the monitoring period. The data was processed, using proprietary software, to determine temperature and barometric pressure compensated water levels.

The equipment can record parameters at pre-determined time intervals and has a capacity of many thousands of readings and is therefore suitable for monitoring over extended periods of time.

APPENDIX B

Photographs



B



Overview of site from north-west corner, looking south-east.



Overview of site from north-west corner, looking east.



Overview of site from north-west corner, looking south.



Borehole BH03 Location, looking east.



Borehole BH04 Location, looking west.



Overview of south of site from BH03 location, looking west.



Overview of south of site from BH03 location, looking west.



Overview of south of site from BH03 location, looking north-west.



Overview of south of site from BH03 location, looking north.



Borehole BH01 Location, looking north-east.



Overview of south of site from BH01 location, looking south.



Overview of south of site from BH01 location, looking west.



Borehole BH02 Location, looking west.



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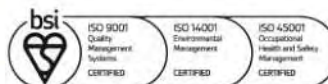


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